

Clark Dietrich. Spazzer® Spacer & Bridging Bar Design Tables

Technical Services: 888-437-3244 Sales: 800-543-7140 Engineering Services: 877-832-3206 clarkdietrich.com

Spazzer®	5400 Spacing	& Bridging Bar - 18GA (S	PZS-43) - Con	nection Stre	ngth and Stif	fness					
Product	Stud Depth (in)	Allowable Capacities	Stud Thickness, mils (ga.)								
Product	Stud Deptil (III)	Allowable Capacities	33 (20)	43 (18)	54 (16)	68 (14)					
		Brace Stiffness (lbs/in)	525	735	1160	1380					
	3-5/8	Brace Strength (lbs)	310	360	360	360					
		Torsional Moment (in-lbs)	150	240	300	300					
		Brace Stiffness (lbs/in)	290	420	520	890					
SPZS-43	6	Brace Strength (lbs)	300	340	340	340					
		Torsional Moment (in-lbs)	210	250	290	290					
		Brace Stiffness (lbs/in)	-	240	430	650					
	8	Brace Strength (lbs)	-	290	320	320					
		Torsional Moment (in-lbs)	-	230	250	280					

Spazzer Bar Allowable Table Notes:

- Allowable loads are based on the use of cold-formed steel studs with a minimum yield strength, Fy=33 ksi and tensile strength, Fu=45 ksi for 43-mil (18-ga) or thinner and a minimum yield strength, Fy=50 ksi and tensile strength, Fu=65 ksi for 54 mil (16-ga) or thicker.
- Allowable loads are based on 43-mil (18-ga) 4300 Spazzer Bar with a minimum yield strength, Fy=33 ksi and tensile strength, Fu=45 ksi.
- Allowable loads are for the bridging connection only. The strength and serviceability of the framing members is the responsibility of the designer.
- Allowable loads may not be increased for wind or seismic load.
- Allowable loads are for use when using ASD design methodology. For LRFD loads, multiply ASD allowable loads by 1.6.
- Allowable brace loads are based on ultimate test loads divided by a safety factor. Serviceability limits are not considered. Brace stiffness requirements are detailed in AISI S100 Section D3.3.
- Axial brace stiffness values apply to both ASD and LRFD designs.
- Listed Spazzer Bar capacities are based on Spazzer Bar fully seated in the bottom of the stud knockout as shown in Figure-1.



Figure-1

Spazzer® 5400 Spacing & Bridging Bar - 18GA (SPZS-43) - Gross Properties															
Product	Design		Area	. I. (in⁴)	S _x (in³)	R _x (in)	l _v (in⁴)	S _v (in³)	R _y (in)	Torsional Properties					
Code	Thickness	F _y (ksi) (in ²)								Jx1000	C _w	Y _o	m	R_o	в
	(in)				(in)							(in⁴)	(in ⁶)	(in)	(in)
SPZS-43	0.0451	33	0.112	0.0026	0.0093	0.1513	0.0470	0.0415	0.6493	0.07563	1.79E-09	0.2622	-0.014	0.716	0.866

Spazzer	Spazzer® 5400 Spacing & Bridging Bar - 18GA (SPZS-43) - Net Properties																
_	Design		Area							Torsional Properties							
Product Code	Thickness F _y (F _y (ksi)		I _x (in⁴)	S _x (in ³)	R _x (in)	I _v (in⁴)	S _v (in ³)	R _y (in)	Jx1000	C _w	Yo	m	Ro			
				-		,		(in²)	(in ⁻)				,		(in⁴)	(in ⁶)	(in)
SPZS-43	0.0451	33	0.066	0.0005	0.0032	0.0906	0.0100	0.0147	0.3877	0.045052	1.01E-09	0.15632	0.093	0.428	0.867		

Spazzer® 5400 (SPZS-43) - Allowable Member Strengths											
Product Code	M _a (F _y) (in-lbs)	M _a (12"o.c.) (in-lbs)	M _a (16"o.c.) (in-lbs)	M _a (24"o.c.) (in-lbs)	P _a (12"o.c.) (lbs)	P _a (16"o.c.) (lbs)	P _a (24"o.c.) (lbs)				
SPZS-43	179	179	179	161	525	485	375				

Net section properties are based on section that excludes material from slot/notch.

[•] Member strengths analysis are based on AISI S100-12.



Clark Dietrich. Spazzer® Spacer & Bridging Bar Design Tables

Technical Services: 888-437-3244 Engineering Services: 877-832-3206 Sales: 800-543-7140 clarkdietrich.com

Spazzer® 5400 Spacing & Bridging Bar - 18GA (SPZS-43) - Maximum Bridging Distance (ft.)

Spazzei	34003	pacing & bri	ugiii	B D	ш - Т	ADO.	(1)P	23-4	3] - I	viaxi	mui	пы
Stud	Stud	Stud thickness,				Lateral	Stud P	ressui	re (psf)		
Spacing, in	Section	mils (ga.)	5	10	15	20	25	30	35	40	45	50
		33 (20)	8	8	5	4						
	2625162	43 (18)	8	8	8	6	5	4	4			
	362S162	54 (16)	8	8	8	8	6	5	4	4		
		68 (14)	8	8	8	8	6	5	4	4		
		33 (20)	8	6	4							
	362S200	43 (18)	8	8	6	5	4					
	3023200	54 (16)	8	8	8	6	5	4				
		68 (14)	8	8	8	6	5	4				
		33 (20)	8	8	8	6	5	4	4			
	600S162	43 (18)	8	8	8	7	6	5	4	4		
12		54 (16)	8	8	8	8	7	6	5	4	4	
		68 (14)	8	8	8	8 5	7	6	5	4	4	4
		33 (20)	8	8	6 7	6	4	4				
	600S200	43 (18)	8	8	8	7	5	4	4			
		54 (16)	8	8	8	7	5	4	4			
		68 (14)	8	8	8	8	6	5	4	4		
	9000163	43 (18)	8	8	8	8	7	6	5	4	4	
	800S162	54 (16)	8	8	8	8	8	6	5	5	4	4
		68 (14)	8	8	8	6	5	4				
	800S200	43 (18)	8	8	8	6	5	4	4			
	8003200	54 (16)	8	8	8	7	6	5	4	4		
		68 (14)	8	6	4							
		33 (20) 43 (18)	8	8	6	5	4					
	362S162	54 (16)	8	8	8	6	5	4				
		68 (14)	8	8	8	6	5	4				
	362S200	33 (20)	8	4								
		43 (18)	8	7	5	4						
		54 (16)	8	8	6	4	4					
		68 (14)	8	8	6	4	4					
		33 (20)	8	8	6	5	4					
		43 (18)	8	8	7	6	4	4				
4.6	600S162	54 (16)	8	8	8	7	5	4	4			
16		68 (14)	8	8	8	7	5	4	4			
	600S200	33 (20)	8	7	5							
		43 (18)	8	8	6	4						
		54 (16)	8	8	7	5	4					
		68 (14)	8	8	7	5	4					
		43 (18)	8	8	8	6	5	4				
	800S162	54 (16)	8	8	8	6	5	4	4			
		68 (14)	8	8	8	7	6	5	4	4		
		43 (18)	8	8	6	4						
	800S200	54 (16)	8	8	6	5	4					
		68 (14)	8	8	7	5	4	4				
		33 (20)	8	4								
	362S162	43 (18)	8	6	4							
		54 (16)	8	8	5	4						
		68 (14)	8	8	5	4						
		33 (20)	6	_								
	362S200	43 (18)	8	5								
		54 (16)	8	6	4							
		68 (14)	8	6	4							
		33 (20)	8	6 7	5	4						
	600S162	43 (18)		8		4						
24		54 (16)	8	8	6 6	4	4					
		68 (14)	8	5								
		33 (20)	8	6	4							
	600S200	43 (18)	8	7	4							H
		54 (16)	8	7	4							
		68 (14)	8	8	5	4						
	800S162	43 (18)	8	8	6	4						
	8003102	54 (16) 68 (14)	8	8	6	5	4					
			8	6	4							
	800S200	43 (18) 54 (16)	8	6	4							
	0003200	68 (14)	8	7	5	4						
		00 (14)										

- Tabulated maximum bridging distances are for ASD lateral pressures.
- Tabulated maximum bridging distances are based on the tested connection strength.
- Studs must be checked for unbraced length seperately.
- Lateral pressures shall be determined based on the load combinations of the applicable building code.
- For designs using 2009 IBC and earlier, wind pressures are at the working stress level and may be used directly.
- For designs using 2012 IBC and 2015 IBC, wind pressures are at the strength level and must be multiplies by 0.6 for ASD load combinations.



Clark Dietrich. Spazzer® Bridging & Spacer Bar Design Tables

Technical Services: 888-437-3244 Engineering Services: 877-832-3206 Sales: 800-543-7140 clarkdietrich.com

Spazzer® 5400 Spacing & Bridging Bar - 18GA (SPZS-43) - Design Example

Example-1: Exterior Bearing-Wall Stud

Input

- 2012 IBC (ASCE 7-10 & AISI S100-2012)
- -600S162-43 (33-ksi) studs at 16" o.c., 10 ft. tall
- Bracing at 5-ft o.c. (Mid-point bracing)
- Nominal axial stud strength, Pn=5400 lbs (2013 AISI Manual, Table III-8)
- Distance from shear center to mid-plane of web, m=0.670-in (2013 AISI Manual, Table I-2)
- Wind Design Pressure = 20psf

Laterally-Loaded Stud Design

Design Load tributary to brace:

W=(0.6)(20psf)(16"/12")(5ft) = 80 lbs

Note-IBC 2015 load combinations for ASD include a factor of 0.6 for wind loads.

Required flange force (AISI S100 Eq. D3.2.1-3)

P = 1.5(m/d)W = 1.5(0.67/6)80 = 10.05 lbs

Torsional Moment

 $M_z = P(d) = 10.05(6) = 60.3 \text{ in-lbs}$

Moment applied to bridging member

 $M_m = 0.64(M_z) = 0.64*60.3 = 38.6 \text{ in-lbs}$

(Note: For 0.64 factor, refer AISI Design Guide D110-07 for analysis of a five-span continuous beam that is loaded with equal support moments)

From Allowable Loads Table for 6-in deep 43-mil stud,

Allowable Torsional Moment = 250 in-lbs > 60.3 in-lbs OK

Check member strength from allowable strengths table for 16" o.c.

Allowable moment = 179 in-lbs > 38.6 in-lbs OK

Axially-Loaded Stud Design

Required brace strength (AISI S100 Eq. D3.3-1)

P = 0.01(Pn) = 0.01(5400) = 54 lbs.

For ASD, divide by 1.5 (2012 AISI Cold-Formed Steel Design Manual, Pg. III-54)

(54)/(1.5) = 36.0 lbs.

Required brace stiffness (AISI S100 Eq. D3.3-2)

 $\beta = 2[4-(2/n)](Pn)/(L) = 2[4-(2/1)](5400)/(60) = 360 lbs/in$

From Allowable Loads Table for 6-in deep 43-mil stud,

Allowable brace strength = 340 lbs > 54 lbs. OK Brace stiffness = 420 lbs/in > 360 lbs/in. OK

Combined-Loading Checks

Connection

 $P_{br}/P_{n} + M_{z}/M_{a} \le 1.0$ =54/340+60.3/250=0.40<1.0 OK

Bridging Member

 $\Omega_c P/P_n + \Omega_b M/M_n \le 1.0$

1.8*54/675 + 1.67*38.6/299 = 0.360 < 1.0 OK